

**BENEFIT  
STATEMENT****Case Analysis**

## MARKET SECTOR

**Mining  
Industry**

## APPLICATION

**Thick Lift  
Compaction**

## PROJECT PHASE

**Design  
Phase****maximizing the use of on-site  
materials, eliminating the need to  
import alternative material****PROJECT and  
GEOLOGICAL REVIEW**

This design project refers to the haul road design of a new Uranium mine located near Swakopmund, on the west coast of Namibia.

The project is the largest in-situ and highest grade granite-hosted uranium deposit in Namibia and currently the fifth largest uranium-only deposit in the world.

The area is a semi-arid region with an extremely dry sandy material (desert-like) of very poor quality.

**DESIGN REVIEW and  
ALTERNATIVE PROPOSAL**

The original Haul Road design included the following:

- 1000mm layer of rockfill directly on top of "untreated" in-situ material (G6 equivalent used in the analysis).
- 350mm of a granular material predominantly characterized by rock (G5 equivalent used in the analysis).
- 150mm wearing course (G4 equivalent used in the analysis).

The problem associated with the design was the fact that the rock/granular material was not available on site and would have to be hauled in from a quarry quite a distance away. In addition to the unavailable site material, crushing facilities would be required to control

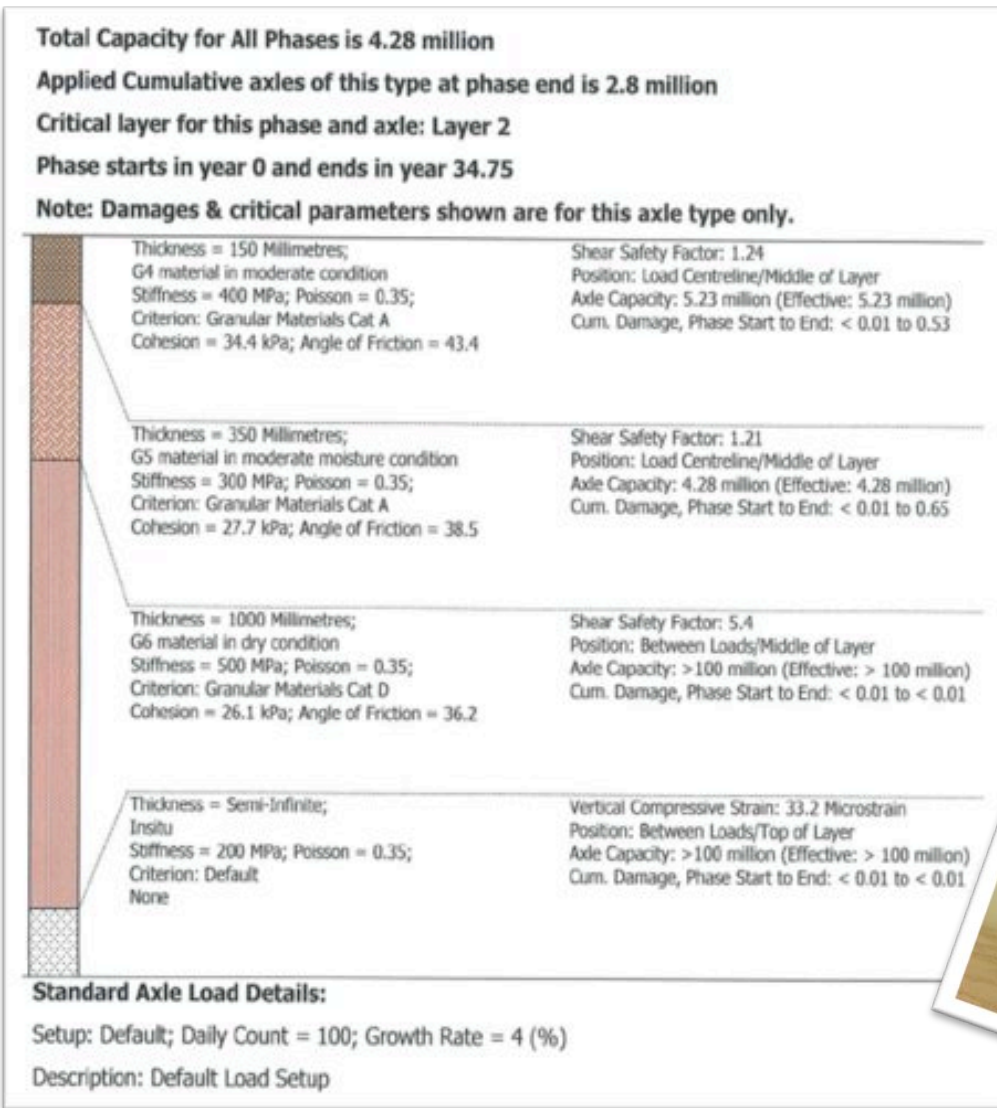
the particle size required for the construction of the layerworks conventionally.

The use of impact compaction would offer great advantages in the construction of the original design in that, with thicker layerworks, minimal crushing would be required, reducing the construction phase of the haul road whilst offering a stronger longer life platform.

With G5 being widely available on site, the benefits of utilising impact compaction using available materials and thick lift compaction became apparent, as depicted in the revised design. Substituting the multiple "granular/rockfill" layers with two single G5 layers, each being 750mm thick, not only saved on material hauling and crushing and pavement construction but also resulting in a longer life pavement when compared under standard axle loading criteria.



ORIGINAL DESIGN ANALYSIS



## ALTERNATIVE DESIGN ANALYSIS

**Total Capacity for All Phases is > 100 million**

**Applied Cumulative axles of this type at phase end is 2.8 million**

**Critical layer for this phase and axle: Layer 1**

**Phase starts in year 0 and ends in year 34.75**

**Note: Damages & critical parameters shown are for this axle type only.**

Thickness = 1500 Millimetres;  
GS material in moderate moisture condition  
Stiffness = 300 MPa; Poisson = 0.35;  
Criterion: Granular Materials Cat A  
Cohesion = 27.7 kPa; Angle of Friction = 38.5

Shear Safety Factor: 3.8  
Position: Between Loads/Middle of Layer  
Axle Capacity: >100 million (Effective: > 100 million)  
Cum. Damage, Phase Start to End: < 0.01 to < 0.01

Thickness = Semi-Infinite;  
Insitu  
Stiffness = 200 MPa; Poisson = 0.35;  
Criterion: Default  
None

Vertical Compressive Strain: 37.5 Microstrain  
Position: Between Loads/Top of Layer  
Axle Capacity: >100 million (Effective: > 100 million)  
Cum. Damage, Phase Start to End: < 0.01 to < 0.01

**SAVE UP TO  
50% OF  
ORIGINAL  
EXPECTED  
COST.**



### **Standard Axle Load Details:**

Setup: Default; Daily Count = 100; Growth Rate = 4 (%)

Description: Default Load Setup

## **SUMMARY**

- Eliminating the need to haul and crush imported material.
- Superior subgrade strength.
- Longer life pavement.
- Construction time savings.
- Substantial cost savings.